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Sediment Budget and Relief Development in Hrafndalur, Subarctic Oceanic Eastern Iceland

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Abstract

There have been only a few truly integrated quantitative studies on sediment budgets and relief development in cold environments. For a combined quantitative investigation of the relevant denudative slope processes and stream work in the 7 km² catchment Hrafndalur, situated in a rhyolite area in the northern part of the Icelandic Eastern Fjords region (Austfirðir), information was collected on the absolute and relative importance of the different denudative processes. Integration of the six-year monitoring program with the analysis of Holocene storage elements using geophysical techniques allows estimates on Holocene process intensities in addition to the measured present-day sedimentary transfers. With respect to mass transfers, fluvial suspended sediment plus bed-load transport is most relevant and is followed by fluvial solute transport, rock and boulder falls, chemical slope denudation, mechanical fluvial slope denudation (slope wash), creep processes, avalanches, debris flows, translational slides, and deflation. Due to comparably high mechanical weathering and sedimentary transfer rates, postglacial modification of the Pleistocene glacially formed landscape is clearly more advanced than in the extended basalt areas of Austfirðir. Postglacial relief development is characterized by a valley widening due to the active retreat of rock walls and the formation of extended talus cones coupled with channel systems.

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Introduction

SEDIMENT BUDGET STUDIES IN COLD ENVIRONMENTS:

There have been only few truly integrated quantitative studies of sediment budgets and relief development in cold environments (e.g. Jäckli, 1957; Rapp, 1960; Caine, 1974, 2004; Caine and Swanson, 1989; Barsch, 1981; Warburton, 1993, 2006; Becht, 1995; Beylich, 2000, 2002, 2008; Beylich et al., 2005; Beylich and Warburton, 2007; Beylich et al., in review; Schrott et al., 2002, 2003; Otto and Dikau, 2004; Slaymaker, 2008; Hansen et al., in press). Projected climate change will cause major changes in Earth surface systems with the most dramatic changes being expected to occur in subpolar and polar environments. Geomorphic processes that operate at the Earth's surface and transfer sediments and change landforms are highly dependent on climate, vegetation cover, and human impact and will be significantly affected by climate change (e.g. Rapp, 1985; Evans and Clague, 1994; Haeberli and Beniston, 1998; Lamoureux, 1999; Lamoureux et al., 2007; Slaymaker et al., 2003; Orwin and Smart, 2004; Beylich et al., 2006b, 2008; Beylich and Warburton, 2007; Cockburn and Lamoureux, 2007; Slaymaker, 2008).

A better knowledge of rates of sedimentary transfer processes operating in present-day climates is needed to determine the consequences of predicted climate change. It is necessary to collect and compare data on both contemporary sedimentary fluxes and on storage elements from a wide range of different high-latitude and high-altitude cold environments and to apply more standardized methods for research on sediment fluxes and relationships between climate and sedimentary transfer processes (Beylich et al., 2006a, 2008; Beylich and Warburton, 2007; Lamoureux et al., 2007). The aims of this study are to: (i) analyze the rates and the spatio-temporal variability of denudative processes and sedimentary transfers within the Hrafndalur catchment; (ii) investigate the absolute and the relative importance of the different denudative processes; (iii) quantify the sediment budget for this catchment; and (iv) analyze trends of relief development in this subarctic oceanic environment.

STUDY AREA

The Hrafndalur catchment (7 km², 6–731 m a.s.l.; 65°28'N, 13°42'W) is situated in the northern part of the Easter Fjords region (Austfirðir) in eastern Iceland (Figs. 1 and 2). The climate is subarctic oceanic, with a mean annual precipitation of 1719 mm yr⁻¹ and mean annual air temperature of 3.6 °C. Runoff occurs year-round with the highest discharges during spring snowmelt (normally April-June), winter thaw events, and especially during extreme rainfall events which are normally most frequent in fall (September-November) (Beylich, 1999, 2000, 2003, accepted). Runoff can cease during dry periods in summer and frost spells in winter (Fig. 3). The steep and glacially sculptured alpine setting contains slopes composed of rock faces and talus cones, and main channels changing between resistance-limited bedrock channels and channel stretches with temporary storage of bed-load material (Fig. 2). The bedrock lithology in Hrafndalur is dominated by rhyolites, while basalt occurs only as smaller intrusions and dikes. Vegetation includes lichens, mosses, meadows, bogs, and dwarf shrubs. Relevant denudative processes are rock and boulder falls, avalanches, debris flows and slides, creep processes, slope wash, chemical denudation, fluvial transport of

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FIGURE 1. Location of the Hrafndalur catchment (Austfirðir, Iceland) and investigation areas, test sites, and instrumentation within the catchment.

solutes, suspended sediments and bed load, and deflation. The main storage elements in the valley are extended talus cones. In addition, there is a Holocene valley fill in the lowest part of Hrafndalur. Human impact exists in the form of grazing which has caused a significant disturbance of the vegetation cover in larger parts of the valley (Fig. 2) (Beylich, 2007).

Approach and Methods

This study focuses on quantifying rates of denudative processes, mass transfers, and the sediment budget of a 7 km^2 catchment. The process rates and mass transfers have been calculated after a six-year investigation (November 2001–Decem-



FIGURE 2. The Hrafndalur catchment in eastern Iceland.

ber 2007) carried out in the Hrafndalur catchment, along with geophysical investigations to quantify volumes of sediment storage elements and regolith thicknesses that were performed in the summer of 2005.

SELECTION OF MEASURING SITES AND SAMPLING SITES

The Hrafndalur catchment is lithologically mainly composed of rhyolites, with some basalt (Beylich, 2007; Beylich et al., 2007a). The selected measurement sites, profiles, slope test sites, and sampling sites/profiles are shown in Figure 1. Figure 2 shows the topography, landforms, and geographical setting of the catchment.

The slope test and sampling sites were selected after analysis of the existing aerial photographs of the area and detailed field investigations. It was our aim to analyze subareas with significant differences in (i) steepness and slope form, (ii) aspect to radiation, (iii) snow cover and ground frost duration, and (iv) vegetation cover.

GEOPHYSICAL INVESTIGATIONS

Geophysical methods are particularly suitable for geomorphologic investigations since the knowledge of structure and composition of the subsurface at different scales is a key parameter for geomorphologic problems. The basic principle for the successful application of geoelectrical methods in geomorphology is based on the varying electric conductivity of minerals, solid bedrock, sediments, air and water, and consequently their varying electrical resistivity. The resistivity of rock, for example, depends on water saturation, chemical properties of pore water, structure of pore volume, and temperature. The large range of resistivity values for most materials is due to varying water content.

Resistivity measurements are made by introducing a direct current into the ground via two electrodes. Resistivity surveys give an image of the subsurface resistivity distribution. Knowing the resistivities of different material types, it is possible to convert the resistivity image into an image of the subsurface consisting of different materials.

For this study the two-dimensional (2D) electrical surveys were performed using the Wenner and Wenner-Schlumberger configurations and an IRIS SYSCAL Junior Switch resistivity meter. In total, 10 selected profiles (profiles a–j in Fig. 1) were performed in summer 2005. The measured apparent resistivities are used to build up a vertical contoured section showing the lateral and vertical variation of resistivity over the section. The conventional method of plotting the results for the interpretation is the so-called pseudosection, which gives an approximate image of the subsurface resistivity distribution. The measured sets of apparent resistivities were inverted using RES2DINV software. Further details on different array geometries and data processing strategies can be found in Reynolds (1997), Kneisel (2003, 2006), and Hauck and Kneisel (2008).

METEOROLOGICAL MEASUREMENTS AND SNOW SAMPLING

Meteorological data are available from a meteorological station (Desjamyri) located close to the lower part of Hrafndalur



FIGURE 3. Daily air temperatures, precipitation, runoff, and daily suspended sediment yields (SSY) and solute yields (SY) in Hrafndalur (catchment outlet) in 2003.

and operated by the Icelandic Meteorological Office (Veðurstofa Islands) (Fig. 1). Precipitation has been measured daily since 1998 in accordance with the regulations of Veðurstofa Islands (1.5 m above ground).

Snow cores of the complete vertical snow pack were taken six times during the entire period of investigation (November 2001–

December 2007) along selected profiles (see Fig. 1) using a plastic tube (10 cm diameter). The snow cores were melted in buckets at the field base and the water was filtered with a pressure filter and ashfree filter papers (Munktell quantitative filter papers, OOH) with the goal to quantify Total Dissolved Solids (TDS) and the concentration of mineralogenic particles in the snow cores (see below).

MEASUREMENT OF RELEVANT DENUDATIVE SLOPE PROCESSES

A combination of process monitoring and geophysical mapping as well as further field observations and detailed photo documentation were used to analyze relevant denudative slope processes and sedimentary fluxes in Hrafndalur. Eight slope test sites (1-8) within the 7 km² catchment were selected after studying aerial photographs and previous fieldwork. The slope test sites differ with respect to slope form, aspect, elevation, and local geographical setting and were selected to cover the range of different settings found within Hrafndalur. Six slope test sites (slope test sites 2 + 4–8) include an active rock face, a talus cone, as well as areas with avalanches, debris flows, slow mass movements, and slope wash. At two slope test sites (slope test sites 1 + 3) there is no rock face left due to Holocene rock wall back-wearing and talus cone formation.

All slope test sites were instrumented in November 2001 to monitor rock and boulder falls (slope test sites 2 + 4-8) as well as creep processes and slope wash (slope test sites 1-8). Additionally, avalanches, debris flows, and slides were studied at selected and other adequate sites within the catchment (Fig. 1).

ROCK FALLS AND BOULDER FALLS

Rock falls and boulder falls were investigated at the six slope test sites with active rock faces (slope test sites 2 + 4-8) by applying a combination of monitoring and detailed photo documentation. At all investigated sites the rock fall and boulder fall activity was locally not influenced by avalanche activity. The denudative importance of avalanches was studied at selected sites within Hrafndalur (see below). At each investigated slope test site an 80 m² (8 m \times 10 m; size of meshes: 1.5 cm) large net was installed with the long axis placed parallel to the face on the talus cone below vertical rock walls (Fig. 1). The nets were efficient in collecting debris produced by mechanical weathering at the rock walls and transferred to the net by primary and secondary rock falls. The collected debris was repeatedly quantified by weighing it with a portable field balance (kg m⁻², kg m⁻² yr⁻¹). Rock wall retreat rate (mm yr^{-1}) was calculated by estimating the surface area of the defined and debris-supplying rock face and relating it to the mass of debris accumulated below the rock face, using an estimated mean rock density of 2.2 (Beylich et al., 2007a). The mass of accumulated debris smaller than ca. 1.0 cm in diameter was quantified with painted rock faces. At each slope test site with debris-supplying rock face two squares of 1 m² were painted in November 2001 and repainted in each following year. Fine debris accumulated below the painted squares could be identified on the debris and the total mass of fine debris was quantified by weighing the debris with a portable field balance. The total mass of fine debris was then related to the clearly defined source area of 1 m^2 rock surface.

Boulder falls were investigated at the six slope test sites with material supplying rock faces by detecting, mapping, counting, and measuring fresh boulders accumulated below the boulder-supplying rock walls. For detecting and mapping of fresh boulder falls, detailed photo documentation was carried out each summer by taking photographs. The boulder size measurements (a-, b-, and c-axes) were carried out in the field.

Three geoelectrical profiles (see profiles f-h in Fig. 1) were also performed on talus cones in order to infer the scree thicknesses.

AVALANCHES

Avalanches occur during the early summer season (normally during April–June) and during winter thaw events. In Hrafndalur, avalanches are restricted to the steep NE-facing valley slope (Figs. 1 and 2). Avalanches triggered at this valley side cause rock and boulder falls at the extended rock faces of this slope (Fig. 2) and transfer both newly released material from rock faces as well as remobilized material from talus cones down-slope. Especially in the upper parts of the catchment, avalanches transfer material from slopes into main channels.

Total annual accumulations (t yr^{-1}) of inorganic material (including fine material, debris, and boulders) by avalanches occurring at the NE-facing valley slope were quantified for the years 2002-2007 by combining a detailed sampling, measuring, and weighing of newly deposited material with an estimation of the entire affected deposition area at the valley slope and a detailed photo documentation of the entire valley slope system. Newly accumulated and dried debris was weighed at defined 1 m² plots within the accumulation areas of avalanches and fine material was sampled for the quantification of inorganic mass (burning of the material over 12 h at 550 °C in the laboratory). Mapping of the deposition area as well as the detection and mapping of fresh boulder falls were carried out each summer during the six-year investigation period. Boulders were measured in field (a-, b-, c-axes). The mean density of boulders was estimated to be 2.2 t m³. (Beylich et al., 2007a).

DEBRIS FLOWS AND SLIDES

Debris flows and debris slides were investigated by detailed annual photographic documentation of slopes. Both new and old traces of debris flows and debris slides were mapped. The volumes of transferred material as well as the transport distances were measured in the field. Debris flows are significant for transferring material from slope to main channel systems.

SLOPE CREEP PROCESSES

Slope creep processes were analyzed at the eight different test sites by monitoring movements of painted stone tracer lines and steel rod lines. At each slope test site three stone lines (A, B, C) with 40 painted stones at each line (A1–A40, B1–B40, C1–C40) were installed in November 2001. The stone tracer lines A–C were installed parallel at defined distances from the rock face of slope test sites 2 and 4–8 and from defined stable rock outcrops at slope test sites 1 and 3. The horizontal distance between stones was one meter. Down-slope movements of all painted stones (120 stones per slope test site) were measured each year with a steel tape.

At alternate locations where a painted stone was placed, a steel rod (1.0 cm diameter) was installed vertically 10 cm down into the ground. In total, 60 steel rods (A1–A20, B1–B20, C1–C20) per slope test site were installed. Down-slope movements of all steel rods were measured each year along with the movements of painted stones.

ESTIMATING THE IMPORTANCE OF DEFLATION

The importance of deflation and eolian deposition were estimated by analyzing sediment concentrations in snow cores collected along defined profiles (snow sampling profiles I–VI) within the catchment (see Figs. 1 and 2) (see above).

 TABLE 1

 Rock falls, boulder falls, and rock wall retreat rates at slope test sites in Hrafndalur (2001–2007).

		STS 1	STS 2	STS 2	STS 4	STS 5	STS 6	STS 7	STC 0
		5151	515.2	515.5	5154	515.5	515.0	515 /	515 8
2002	Debris (kg m ⁻²)	_	10.2	_	12.5	12.1	10.8	13.5	10.1
	Retreat (mm yr^{-1})	—	2.3		2.6	2.3	2.5	2.8	2.2
2003	Debris (kg m ⁻²)	_	11.4	—	12.4	11.2	10.7	12.9	9.8
	Retreat (mm yr^{-1})	—	2.6		2.6	2.1	2.4	2.7	2.1
2004	Debris (kg m ⁻²)	—	9.7		11.9	12.2	11.3	13.7	9.5
	Retreat (mm yr^{-1})	—	2.2		2.5	2.3	2.6	2.8	2.1
2005	Debris (kg m ⁻²)	—	10.5		12.0	10.9	11.6	12.8	10.3
	Retreat (mm yr^{-1})	—	2.4		2.5	2.1	2.6	2.6	2.2
2006	Debris (kg m ⁻²)	—	12.3		14.4	12.6	11.5	14.6	11.4
	Retreat (mm yr^{-1})	—	2.8		3.0	2.4	2.6	3.0	2.5
2007	Debris (kg m ⁻²)	—	10.6	_	13.1	11.8	10.9	12.9	10.2
	Retreat (mm yr^{-1})	—	2.4		2.7	2.2	2.5	2.7	2.2
November 2001–December 2007	Mean retreat (debris on nets) (mm yr^{-1})	—	2.5	_	2.7	2.2	2.5	2.8	2.2
	Mean retreat (painted debris) (mm yr ⁻¹)	—	0.2		0.3	0.2	0.2	0.4	0.2
2001-2007	Mean retreat (boulder falls)	—	0.7		0.9	0.4	1.1	1.0	0.8
2001–2007	Mean retreat (total) (mm yr^{-1})	_	3.4	_	3.9	2.8	3.8	4.2	3.2

CHEMICAL DENUDATION, SLOPE WASH, AND FLUVIAL TRANSPORT

Chemical denudation was investigated by analyzing water samples collected from small creeks and soil pipes on the slopes (Figs. 1 and 2). Solute yields and chemical denudation rates for the Hrafndalur catchment were calculated based on measurements of atmospheric solute inputs to the catchment, runoff, and solute concentrations in the main creek (see below). Water chemistry analyses were performed in the laboratory of the Geological Survey of Norway (NGU) in Trondheim, using ICP-AES (inductively coupled plasma atomic emission spectroscopy) and IC (Dionex Ioncromatograph 120 DX).

Slope wash was studied with Gerlach traps installed at the eight slope test sites within Hrafndalur. In addition, suspended sediment concentrations in small creeks draining the slope systems were analyzed (Figs. 1 and 2) (see below). Discharge at the outlet of the catchment was measured by continuous and year-round monitoring of water level using a pressure sensor (GLOBAL WATER) recorded at hourly intervals, in combination with 225 propeller measurements using an Ott-propeller (model C2) at selected water level stages between 2001 and 2007. Daily specific runoff (mm d⁻¹) was calculated by dividing calculated daily discharge by the catchment area (Beylich, 1999). The stable channel cross-sectional profile at the bridge where measurements were taken provided reliable measurements and a strong correlation between water level and discharge (r = 0.92, n = 225).

Fluvial suspended sediment and solute transport were analyzed by combining continuous and year-round monitoring of turbidity and electric conductivity (GLOBAL WATER) with

TABLE 2

Annual mass transfers by avalanches in Hrafndalur (2002–2007).

Annual mass (t) Avalanches Mean distance (m) Avalanches Annual mass transfer (t m) Avalanches 2002 36 202 7272 2003 45 233 10485 2004 50 252 12600 2005 38 196 7448 2006 41 282 11562 2007 52 241 12532 2002-2007 10317				
2002 36 202 7272 2003 45 233 10485 2004 50 252 12600 2005 38 196 7448 2006 41 282 11562 2007 52 241 12532 2002–2007 10317		Annual mass (t) Avalanches	Mean distance (m) Avalanches	Annual mass transfer (t m) Avalanches
2003 45 233 10485 2004 50 252 12600 2005 38 196 7448 2006 41 282 11562 2007 52 241 12532 2002-2007 10317	2002	36	202	7272
2004 50 252 12600 2005 38 196 7448 2006 41 282 11562 2007 52 241 12532 2002–2007 10317	2003	45	233	10485
2005 38 196 7448 2006 41 282 11562 2007 52 241 12532 2002–2007 10317	2004	50	252	12600
2006 41 282 11562 2007 52 241 12532 2002–2007 10317	2005	38	196	7448
2007 52 241 12532 2002-2007 10317	2006	41	282	11562
2002–2007 10317	2007	52	241	12532
	2002-2007			10317

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hourly readings with discrete water sampling during the field campaigns (2001–2007). In total 445 water samples were collected at the catchment outlet to calculate the relationship between turbidity and suspended sediment concentration. Vertically integrated water samples were taken with 1000 mL wide-necked polyethylene bottles. The samples were filtered in the field with a pressure filter and ash-free filter papers (Munktell quantitative filter papers). After the field campaigns, the filter papers were burned (550 °C) to analyze the concentrations of minerogenic suspended solids (mg L⁻¹). For the relationship between turbidity and suspended sediment concentration the correlation coefficient of r = 0.89 (n = 445) indicates a high reliability for the calculation of annual suspended sediment yields based on continuous, hourly turbidity measurements.

The estimation of annual solute yields was based on the relationship between electrical conductivity and concentration of total dissolved solids. This relationship was calculated after using a total of 128 water samples. The correlation between measured electric conductivity and analyzed concentration of total dissolved solids of these water samples was strong (r = 0.91, n = 128), and calculation of annual yields of solutes based on continuous hourly measurements of electric conductivity at the catchment outlet appears to be reliable.

The stability of creeks and channel stone pavements as well as the range of bed-load transport was estimated by using painted stone tracer lines at selected creeks and channel stretches (Fig. 1). In addition, fresh accumulations of debris/bed load were analyzed by weighing of debris (portable field balance) and by a detailed measuring of the volumes of fresh deposits. The estimation of annual bed-load transport rates presented in this paper might include larger errors, but the repeated analysis (two to three times per years) of the channel stretches above and below the catchment outlet using photo documentation and the measuring of the volumes of fresh bed-load transport must be included in the annual sediment budget estimations given that bed-load transport plays an important role in Hrafndalur.

In addition to the continuous and year-round monitoring and frequent samplings at the catchment outlet, surface water electric conductivity, corrected to 25 $^{\circ}$ C, was measured at different locations within Hrafndalur (Figs. 1 and 2) using a portable conductivity meter.

 TABLE 3

 Annual mass transfers by debris flows and translational slides in Hrafndalur (2002–2007).

	Annual mass (t) Debris flows	Mean distance (m) Debris flows	Mass transfer (t m) Debris flows	Annual mass (t) Translational slides	Mean distance (m) Translational slides	Annual mass transfer (t m) Translational slides
2002	35.4	100	3540	9.3	22	205
2003	20.6	133	2740	7.8	15	117
2004	45.3	86	3896	12.5	36	450
2005	25.4	123	3124	6.4	11	70
2006	27.8	142	3948	9.4	41	385
2007	34.2	96	3283	12.6	28	353
2002-2007			3422			263

Results

ROCK FALLS AND BOULDER FALLS

Masses of newly accumulated debris (kg m^{-2}) on nets installed below rock faces, corresponding to annual rock wall retreat rates (mm yr^{-1}) and mean annual rock wall retreat rates $(mm yr^{-1})$ during the study period (2001–2007) at the slope test sites 2 and 4-8 are presented in Table 1. Rock fall activity is caused by freeze-thaw cycles including (i) annual cycles, (ii) frostfree days following frost periods of several days duration, as well as (iii) daily freeze-thaw cycles. Most boulder falls are triggered by avalanches. The highest frequency of rock and boulder falls is during spring (April/May). Strong wind gusts and heavy rainfall can also trigger secondary summer rock falls (Beylich, 1999, 2003). The mean annual rock wall retreat rates vary between 2.8 mm yr $^{-1}$ at slope test site 5 and 4.2 mm yr $^{-1}$ at slope test site 7. The ratio of the largest recorded annual rate and the smallest annual rate is accordingly 1.5. The mean of all six recorded mean annual rock wall retreat rates is 3.6 mm yr^{-1} .

The results of three geoelectrical profiles undertaken on talus cones in the upper part of Hrafndalur are shown in Figure 4. The images of resistivities against depth show a general decrease in resistivity with depth with a clear contrast between higher resistivities in the upper 15–20 m and higher conductivity beneath a depth of 25 m. The higher resistivities in the upper parts are due to the coarser scree material of the talus cone, while the improved conductivity is due to finer-grained weathered regolith at depth. From the results of the geoelectrical surveys a thickness of the talus cones of 15–20 m can be inferred.

With a mapped surface area of 1.8 km^2 for talus cones (see Table 9 below) a total postglacial volume of rock fall debris of about 31.5 million m³ or ca. 63 million tons can be estimated. This corresponds to a mean annual rock wall retreat rate of ca. 4 mm yr⁻¹ over the last 8000 years (with a given total rock wall surface area of 0.9 km^2 ; see Table 9 below), which is slightly higher than the measured rate of 3.6 mm yr⁻¹ during the last six years. It has to be considered that a relevant amount of material is transported from talus cones into main channels especially since talus cones were reaching a size where they can feed directly into main channels (see below). Accordingly, the postglacial rate of rock wall retreat can be expected to be slightly higher than 4 mm yr⁻¹. This would also be consistent with the assumption of higher process rates directly after the deglaciation (e.g. Ballantyne, 2002).

AVALANCHES

Annual masses of inorganic material deposited by avalanches (t), mean transport distances (m), and corresponding annual mass transfers (t m) caused by avalanches are provided in Table 2. The

mean annual mass of deposited inorganic material is 43.7 t, which is composed of boulders, debris, and finer material. The largest annual deposition occurred in 2007 (52 t) and the smallest in 2002 (36 t).

DEBRIS FLOWS AND SLIDES

Debris flows and translational slides in Hrafndalur are triggered by heavy rainfall (Beylich, 2003). Annual masses, transport distances, and mass transfers connected to debris flows and translational slides are shown in Table 3. For the period 2002–2007 mass transfers by debris flows (3422 t m) are 13 times higher than mass transfers caused by translational slides (263 t m).

CREEP PROCESSES

Mean annual movement rates of stone tracer lines as monitored at the eight slope test sites over the six-years investigation period within the Hrafndalur catchment are given in Table 4. Movements of stone tracers are mainly caused by (i) avalanches and (ii) hits by rock falls, also reflected in significantly higher mean annual movement rates of entire stone tracer lines at the slope test sites, which are composed of an active rock face and a talus cone (slope test sites 2 and 4-8), compared to the slope test sites without an active rock face (slope test sites 1 and 3). This assumes that both avalanches and rock falls cause a significant redistribution of older material on talus cones, in addition to supplying and accumulating new material. The mean annual movement rates of entire stone tracer lines at slope test sites with avalanche and rock fall activity varied from 19.8 cm yr⁻¹ at slope test site 5 (stone tracer line C) to 33.7 cm yr^{-1} at slope test site 7 (stone tracer line A). At the two slope test sites without any avalanche and rock fall activity the mean annual movement rates of entire stone tracer lines range from 7.3 cm yr^{-1} at slope test site 3 (stone tracer line B) to 13.9 cm yr^{-1} at slope test site 1 (stone tracer line B). In addition to avalanches and rock falls, stone tracer movements are also caused by freeze-thaw events, extreme rainfall events causing saturation overland flow (see below), and by strong wind gusts.

Table 4 also presents mean annual movement rates of steel rod lines at the eight slope test sites. The movement rates at the slope test sites without sediment delivery from active rock faces (slope test sites 1 and 3) range from 0.9 cm yr^{-1} at slope test site 1 (steel rod lines B and C) to 1.2 cm yr^{-1} at slope test site 2 (steel rod line A) and are significantly smaller than at the sites with active rock faces (slope test sites 2 and 4–8) where rates between 2.4 cm yr⁻¹ at slope test sites 4 (steel rod lines B and C) and 8 (steel rod line C) and 3.0 cm yr⁻¹ at slope test site 7 (steel rod line A) were measured. The down-slope movement rates of the steel rod lines clearly depend on the rates of sediment supply from

TABLE 4

Mean annual movement rates of painted stone tracer lines and steel rod lines over six years at eight slope test sites (1–8) in Hrafndalur (2001–2007).

		Mean annual movement
Slope test site and	Number of painted	rates of entire stone
tracer lines	stone tracers	tracer lines (cm yr^{-1})
tracer lines	stone tracers	tracer lines (cill yr)
1 A	40	10.2
1 R	40	13.0
	40	13.9
IC	40	12.2
1A, 1B, 1C	120	12.1
2 A	40	28.4
2 B	40	32.0
2.0	40	32.9
20	40	29.4
2A, 2B, 2C	120	30.2
3 A	40	8.5
3 B	40	73
3.6	10	0.7
30	40	9.7
3A, 3B, 3C	120	8.5
4 A	40	26.5
4 B	40	27.9
4.0	40	26.8
40	40	20.8
4A, 4B, 4C	120	2/.1
5 A	40	22.2
5 B	40	23.5
5 C	40	19.8
5 4 5 D 5 C	120	12.0
<i>JA, JB, JC</i>	120	21.8
6 A	40	25.2
6 B	40	24.0
6 C	40	24.6
61 6B 6C	120	24.6
0A, 0B, 0C	120	24.0
/ A	40	33.7
7 B	40	30.4
7 C	40	31.1
7A 7B 7C	120	31.7
0 4	120	26.0
8 A	40	26.0
8 B	40	25.4
° C	40	27.8
8 C	40	27.0
8 C 8A, 8B, 8C	120	27.8 26.4
8 C 8 A, 8 B, 8 C	120	27.8 26.4
8 C 8A, 8B, 8C	120	27.6 26.4 Mean annual movement
8 C 8A, 8B, 8C Slope test site and steel rod	120	21.0 26.4 Mean annual movement rates of entire steel rod
<i>8A, 8B, 8C</i> Slope test site and steel rod lines (10 cm denth)	120	26.4 Mean annual movement rates of entire steel rod lines (cm vr $^{-1}$)
Slope test site and steel rod lines (10 cm depth)	120 Number of steel rods	21.0 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹)
Slope test site and steel rod lines (10 cm depth)	120 Number of steel rods 20	21.0 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0
$\frac{8A, 8B, 8C}{\text{Slope test site and steel rod}}$ $\frac{1}{1} \frac{A}{1}$	120 Number of steel rods 20 20	21.0 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9
Slope test site and steel rod lines (10 cm depth) 1 A 1 B	Number of steel rods	21.0 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9
Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C	120 Number of steel rods 20 20 20 20	21.6 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9
Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C 1A, 1B, 1C	120 Number of steel rods 20 20 20 60	27.6 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 0.9 0.9
Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C 1A, 1B, 1C 2 A	120 <u>120</u> Number of steel rods 20 20 20 60 20 20	21.0 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 0.9 2.9
<i>8</i> C <i>8A, 8B, 8C</i> Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C <i>1A, 1B, 1C</i> 2 A 2 B	120 <u>120</u> Number of steel rods 20 20 20 60 20 20 20 20 20 20 20 20 20 2	27.6 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 0.9 2.9 2.8
<i>8</i> C <i>8A, 8B, 8C</i> Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C <i>IA, 1B, 1C</i> 2 A 2 B 2 C	120 <u>120</u> <u>120</u> 20 20 20 60 20 20 20 20 20 20 20 20 20 2	21.0 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 0.9 2.9 2.8 2.9 2.8 2.9
<i>8</i> C <i>8A, 8B, 8C</i> Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C <i>1A, 1B, 1C</i> 2 A 2 B 2 C 2 <i>4, 2B, 2C</i>	120 120 Number of steel rods 20 20 20 20 20 20 20 20 20 20	27.6 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 0.9 2.9 2.8 2.9 2.8 2.9 2.0
<i>8 C</i> <i>8A, 8B, 8C</i> Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C <i>1A, 1B, 1C</i> 2 A 2 B 2 C <i>2A, 2B, 2C</i>	120 <u>120</u> <u>120</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u>	27.8 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 0.9 2.9 2.8 2.9 2.8 2.9 2.9 2.9 2.9 2.9 2.5 2.9 2.9 2.9 2.9 2.9 2.9 2.9 2.9
<i>8C</i> <i>8A, 8B, 8C</i> Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C <i>1A, 1B, 1C</i> 2 A 2 B 2 C <i>2A, 2B, 2C</i> 3 A	120 <u>120</u> <u>120</u> 20 20 20 60 20 20 20 20 60 20 20 20 20 20 20 20 20 20 2	21.0 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 0.9 2.9 2.8 2.9 2.8 2.9 2.9 2.8 2.9 2.9 1.2
<i>8</i> C <i>8A, 8B, 8C</i> Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C <i>1A, 1B, 1C</i> 2 A 2 B 2 C <i>2A, 2B, 2C</i> 3 A 3 B	120 <u>120</u> <u>120</u> 20 20 20 20 20 20 20 20 20 20	21.0 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 0.9 2.9 2.8 2.9 2.8 2.9 2.8 2.9 1.2 1.1
<i>8</i> C <i>8A, 8B, 8C</i> Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C <i>IA, 1B, 1C</i> 2 A 2 B 2 C 2 <i>A, 2B, 2C</i> 3 A 3 B 3 C	120 Number of steel rods 20 20 20 20 20 20 20 20 20 20	21.0 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 0.9 2.9 2.8 2.9 2.8 2.9 2.9 2.9 1.2 1.1 1.1
<i>8C</i> <i>8A, 8B, 8C</i> Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C <i>IA, 1B, 1C</i> 2 A 2 B 2 C <i>2A, 2B, 2C</i> 3 A 3 B 3 C <i>3A, 3B, 3C</i>	120 120 Number of steel rods 20 20 60 20 20 20 20 60 20 20 20 20 20 20 20 20 20 2	21.6 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 0.9 2.9 2.8 2.9 2.8 2.9 2.9 1.2 1.1 1.1 1.1 1.1
<i>8</i> C <i>8A, 8B, 8C</i> Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C <i>1A, 1B, 1C</i> 2 A 2 B 2 C <i>2A, 2B, 2C</i> 3 A 3 B 3 C <i>3A, 3B, 3C</i> 4 A	120 <u>120</u> <u>120</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u> <u>20</u>	21.8 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 0.9 2.9 2.8 2.9 2.8 2.9 2.9 2.8 2.9 2.9 1.2 1.1 1.1 1.1 2.5
<i>8C</i> <i>8A, 8B, 8C</i> Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C <i>1A, 1B, 1C</i> 2 A 2 B 2 C <i>2A, 2B, 2C</i> 3 A 3 B 3 C <i>3A, 3B, 3C</i> 4 A	20 20	21.8 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 0.9 2.9 2.8 2.9 2.8 2.9 2.9 1.2 1.1 1.1 1.1 1.1 2.5
<i>8</i> C <i>8A, 8B, 8C</i> Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C <i>1A, 1B, 1C</i> 2 A 2 B 2 C <i>2A, 2B, 2C</i> 3 A 3 B 3 C <i>3A, 3B, 3C</i> 4 A 4 B	20 120 Number of steel rods 20	27.8 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 0.9 2.9 2.8 2.9 2.8 2.9 2.9 1.2 1.1 1.1 1.1 1.1 2.5 2.4
<i>8C</i> <i>8A, 8B, 8C</i> Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C <i>IA, 1B, 1C</i> 2 A 2 B 2 C 2 <i>A, 2B, 2C</i> 3 A 3 B 3 C <i>3A, 3B, 3C</i> 4 A 4 B 4 C	120 Number of steel rods 20 20 20 20 20 20 20 20 20 20	21.0 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 2.9 2.8 2.9 2.9 2.9 2.9 2.9 1.2 1.1 1.1 1.1 1.1 2.5 2.4 2.4
<i>8C</i> <i>8A, 8B, 8C</i> Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C <i>1A, 1B, 1C</i> 2 A 2 B 2 C <i>2A, 2B, 2C</i> 3 A 3 B 3 C <i>3A, 3B, 3C</i> 4 A 4 B 4 C <i>4A, 4B, 4C</i>	120 <u>120</u> <u>120</u> 20 20 20 20 20 20 20 20 20 20	21.6 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 2.9 2.8 2.9 2.9 2.8 2.9 2.9 1.2 1.1 1.1 1.1 1.1 2.5 2.4 2.4 2.4 2.4
S C 8A, 8B, 8C Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C 1A, 1B, 1C 2 A 2 B 2 C 2A, 2B, 2C 3 A 3 B 3 C 3A, 3B, 3C 4 A 4 B 4 C 4A, 4B, 4C 5 A	40 120 120 20	21.8 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 2.9 2.8 2.9 2.8 2.9 2.9 2.8 2.9 2.9 1.2 1.1 1.1 1.1 1.1 2.5 2.4 2.4 2.4 2.0
<i>8A</i> , <i>8B</i> , <i>8C</i> Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C <i>1A</i> , <i>1B</i> , <i>1C</i> 2 A 2 B 2 C <i>2A</i> , <i>2B</i> , <i>2C</i> 3 A 3 B 3 C <i>3A</i> , <i>3B</i> , <i>3C</i> 4 A 4 B 4 C <i>4A</i> , <i>4B</i> , <i>4C</i> 5 A	40 120 Number of steel rods 20 <td>27.8 26.4 Mean annual movement rates of entire steel rod lines (cm yr⁻¹) 1.0 0.9 0.9 0.9 2.9 2.8 2.9 2.9 2.9 2.9 2.9 1.2 1.1 1.1 1.1 1.1 2.5 2.4 2.4 2.4 2.4 2.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1</td>	27.8 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 2.9 2.8 2.9 2.9 2.9 2.9 2.9 1.2 1.1 1.1 1.1 1.1 2.5 2.4 2.4 2.4 2.4 2.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1
<i>8C</i> <i>8A, 8B, 8C</i> Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C <i>1A, 1B, 1C</i> 2 A 2 B 2 C <i>2A, 2B, 2C</i> 3 A 3 B 3 C <i>3A, 3B, 3C</i> 4 A 4 B 4 C <i>4A, 4B, 4C</i> 5 A 5 B	20 120 Number of steel rods 20 <td>21.0 26.4 Mean annual movement rates of entire steel rod lines (cm yr⁻¹) 1.0 0.9 0.9 0.9 0.9 2.9 2.8 2.9 2.8 2.9 2.11 1.1 2.5 2.4 2.4 2.0 1.9</td>	21.0 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 0.9 2.9 2.8 2.9 2.8 2.9 2.11 1.1 2.5 2.4 2.4 2.0 1.9
<i>8C</i> <i>8A</i> , <i>8B</i> , <i>8C</i> Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C <i>1A</i> , <i>1B</i> , <i>1C</i> 2 A 2 B 2 C 2 <i>A</i> , <i>2B</i> , <i>2C</i> 3 A 3 B 3 C <i>3A</i> , <i>3B</i> , <i>3C</i> 4 A 4 B 4 C <i>4A</i> , <i>4B</i> , <i>4C</i> 5 A 5 B 5 C	40 120 120 20	21.0 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 2.9 2.8 2.9 2.9 2.9 2.9 2.9 1.2 1.1 1.1 1.1 1.1 2.5 2.4 2.4 2.4 2.4 2.4 2.0 1.9 1.9 1.9
<i>8A</i> , <i>8B</i> , <i>8C</i> Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C <i>1A</i> , <i>1B</i> , <i>1C</i> 2 A 2 B 2 C <i>2A</i> , <i>2B</i> , <i>2C</i> 3 A 3 B 3 C <i>3A</i> , <i>3B</i> , <i>3C</i> 4 A 4 B 4 C 4 A, <i>4B</i> , <i>4C</i> 5 A 5 B 5 C <i>5A</i> , <i>5B</i> , <i>5C</i>	40 120 120 20	21.8 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 0.9 2.9 2.8 2.9 2.9 2.8 2.9 2.9 2.9 2.9 2.9 2.9 2.9 2.9
8A, 8B, 8C Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C 1A, 1B, 1C 2 A 2 B 2 C 2A, 2B, 2C 3 A 3 B 3 C 3A, 3B, 3C 4 A 4 B 4 C 4A, 4B, 4C 5 B 5 C 5A, 5B, 5C 6 A	40 120 120 20	27.8 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 0.9 2.9 2.8 2.9 2.9 2.8 2.9 2.9 1.2 1.1 1.1 1.1 1.1 2.5 2.4 2.4 2.4 2.4 2.4 2.4 2.4 2.4
<i>8</i> C <i>8A, 8B, 8C</i> Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C <i>1A, 1B, 1C</i> 2 A 2 B 2 C <i>2A, 2B, 2C</i> 3 A 3 B 3 C <i>3A, 3B, 3C</i> 4 A 4 B 4 C <i>4A, 4B, 4C</i> 5 A 5 B 5 C <i>5A, 5B, 5C</i> 6 A	40 120 Number of steel rods 20 <td>27.8 26.4 Mean annual movement rates of entire steel rod lines (cm yr⁻¹) 1.0 0.9 0.9 0.9 2.9 2.8 2.9 2.9 2.8 2.9 2.9 1.2 1.1 1.1 1.1 1.1 1.1 1.1 1.1</td>	27.8 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 2.9 2.8 2.9 2.9 2.8 2.9 2.9 1.2 1.1 1.1 1.1 1.1 1.1 1.1 1.1
8A, 8B, 8C Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C 1A, 1B, 1C 2 A 2 B 2 C 2A, 2B, 2C 3 A 3 B 3 C 3A, 3B, 3C 4 A 4 B 4 C 4A, 4B, 4C 5 A 5 B 5 C 5A, 5B, 5C 6 A 6 B C C C C C C C C C	40 120 Number of steel rods 20 <td>21.0 26.4 Mean annual movement rates of entire steel rod lines (cm yr⁻¹) 1.0 0.9 0.9 0.9 2.9 2.8 2.9 2.9 2.9 2.9 2.9 1.2 1.1 1.1 1.1 1.1 1.1 1.1 1.1</td>	21.0 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 2.9 2.8 2.9 2.9 2.9 2.9 2.9 1.2 1.1 1.1 1.1 1.1 1.1 1.1 1.1
8A, 8B, 8C Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C 1A, 1B, 1C 2 A 2 B 2 C 2A, 2B, 2C 3 A 3 B 3 C 3A, 3B, 3C 4 A 4 B 4 C 4A, 4B, 4C 5 A 5 B 5 C 5A, 5B, 5C 6 A 6 B 6 C	40 120 Number of steel rods 20 <td>21.0 26.4 Mean annual movement rates of entire steel rod lines (cm yr⁻¹) 1.0 0.9 0.9 0.9 0.9 0.9 0.9 2.9 2.8 2.9 2.9 2.9 2.9 2.9 2.9 2.9 2.9 2.9 1.1 1.1 1.1 1.1 1.2 1.1 1.9 1.9 1.9 1.9 2.7 2.7 2.6</td>	21.0 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 0.9 0.9 0.9 2.9 2.8 2.9 2.9 2.9 2.9 2.9 2.9 2.9 2.9 2.9 1.1 1.1 1.1 1.1 1.2 1.1 1.9 1.9 1.9 1.9 2.7 2.7 2.6
<i>8A</i> , <i>8B</i> , <i>8C</i> <i>8A</i> , <i>8B</i> , <i>8C</i> Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C <i>1A</i> , <i>1B</i> , <i>1C</i> 2 A 2 B 2 C <i>2A</i> , <i>2B</i> , <i>2C</i> 3 A 3 B 3 C <i>3A</i> , <i>3B</i> , <i>3C</i> 4 A 4 B 4 C <i>4A</i> , <i>4B</i> , <i>4C</i> 5 A 5 B 5 C <i>5A</i> , <i>5B</i> , <i>5C</i> 6 A 6 B 6 C <i>6A</i> , <i>6B</i> , <i>6C</i>	40 120 Number of steel rods 20 <td>27.8 26.4 Mean annual movement rates of entire steel rod lines (cm yr⁻¹) 1.0 0.9 0.9 0.9 2.9 2.8 2.9 2.9 2.8 2.9 2.9 2.9 2.9 2.9 2.9 2.4 2.4 2.4 2.4 2.4 2.4 2.4 2.4</td>	27.8 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 2.9 2.8 2.9 2.9 2.8 2.9 2.9 2.9 2.9 2.9 2.9 2.4 2.4 2.4 2.4 2.4 2.4 2.4 2.4
<i>8</i> C <i>8A, 8B, 8C</i> Slope test site and steel rod lines (10 cm depth) 1 A 1 B 1 C <i>1A, 1B, 1C</i> 2 A 2 B 2 C <i>2A, 2B, 2C</i> 3 A 3 B 3 C <i>3A, 3B, 3C</i> 4 A 4 B 4 C <i>4A, 4B, 4C</i> 5 A 5 B 5 C <i>5A, 5B, 5C</i> 6 A 6 B 6 C <i>6A, 6B, 6C</i> 7 A	40 120 120 20	27.8 26.4 Mean annual movement rates of entire steel rod lines (cm yr ⁻¹) 1.0 0.9 0.9 0.9 0.9 2.9 2.8 2.9 2.9 2.8 2.9 2.9 1.2 1.1 1.1 1.1 1.1 2.5 2.4 2.4 2.4 2.4 2.4 2.4 2.4 2.4

Slope test site and steel rod lines (10 cm depth)	Number of steel rods	Mean annual movement rates of entire steel rod lines (cm yr^{-1})
7 B	20	2.9
7 C	20	2.9
7A, 7B, 7C	60	2.9
3 A	20	2.6
3 B	20	2.5
3 C	20	2.4
8A, 8B, 8C	60	2.5

active rock faces situated above the talus cones, which is, in combination with the down-slope movement rates of material, also controlled by the steepness of the talus cones. The higher the delivery rate, the steeper is the resultant slope angle and the larger is the down-slope movement rate of material. All investigated talus cones are free of vegetation, which also indicates the high rates of mechanical weathering at rock faces and connected rock fall activity and delivery of material. The movement of the monitored 10 cm thick surface layer is mainly caused by the annual freeze-thaw cycle with seasonal ground frost reaching depths more than 0.5 m (Beylich, 1999, 2003).

CHEMICAL SLOPE DENUDATION

Rates of annual atmospheric solute inputs to the catchment, annual solute yields and chemical denudation rates are provided in Tables 5 and 6. The mean annual atmospheric solute input (2002– 2007) in Hrafndalur is 26.0 t km⁻² yr⁻¹ (Table 5). The mean annual chemical denudation rate is 29.3 t km⁻² yr⁻¹ (Table 6). The mean annual discharge-weighted TDS values, ranging from 39.7 mg L⁻¹ in 2002 to 42.0 mg L⁻¹ in 2007 show a low interannual variability. Accordingly annual yields of solutes are strongly dependent on annual runoff, which varied between 1112 mm yr⁻¹ in 2004 (yield of solutes 46.8 t km⁻² yr⁻¹) and 1779 mm yr⁻¹ in 2002 (yield of solutes 70.6 t km⁻² yr⁻¹) (Table 6).

SLOPE WASH

Mean annual slope wash (2002–2007) ranged from 0.008 kg m⁻² (slope test sites 2, 4, 6, and 8) to 0.013 kg m⁻² (slope test site 3). Slope wash occurs during snowmelt and during heavy rainfalls causing saturation overland flow.

Suspended sediments are fluvially transferred from the slopes into the main channels via small creeks, and the annual yields of suspended sediments determined for the outlet of Hrafndalur can be used as estimation of the suspended sediment yields at the slope systems in Hrafndalur. Annual yields of suspended sediments vary significantly from 11.4 t km⁻² yr⁻¹ in 2004 to 30.6 t km⁻² yr⁻¹ in 2002. The mean annual suspended sediment yield for the investigation period (2002–2007) is 18.5 t km⁻² yr⁻¹, which is lower than chemical slope denudation rates in the valley (Table 6). Most suspended sediment transport from the slopes to the channels occurs during heavy rainfalls, which trigger debris flows and cause saturation overland flow at the slope systems (see below).

DEFLATION

Concentrations of inorganic solids in snow cores collected along different selected profiles within Hrafndalur (Figs. 1 and 2)

 TABLE 5

 Annual atmospheric solute inputs in Hrafndalur (2002–2007).

Year	Type of sample	Number of samples	Mean TDS (mg L^{-1})	Total precipitation (mm yr ⁻¹)	Atmospheric solute input (t $\text{km}^{-2} \text{ yr}^{-1}$)
2002	Snow cores	60	15.2	2281	34.7
	Rain water	45			
2003	Snow cores	60	15.5	1666	25.8
	Rain water	50			
2004	Snow cores	60	14.9	1390	20.7
	Rain water	42			
2005	Snow cores	60	15.0	1626	24.4
	Rain water	47			
2006	Snow cores	60	14.8	1869	27.7
	Rain water	44			
2007	Snow cores	60	15.4	1483	22.8
	Rain water	48			
2002-2007				1719	26.0

document that deflation takes place to a certain degree in areas with destroyed, disturbed, or stressed vegetation cover, especially when these areas are exposed in winter due to eolian redistribution of snow. Deflation is significantly increased by needle ice formation in such exposed slope areas (Beylich, 2000, 2003). With respect to mass transfers, deflation has only limited importance in Hrafndalur (see Table 9 below).

FLUVIAL TRANSPORT

Fluvial solute transport, suspended sediment transport, bedload transport, as well as yields of solutes and suspended sediments were quantified for the years 2002–2007 at the outlet of the Hrafndalur catchment (Figs. 1 and 2). Transport rates and yields are presented in Tables 6 and 7. Figure 3 shows daily air temperatures, precipitation, runoff, as well as daily suspended sediment yields and solute yields in the year 2003.

Based on the investigations carried out between 2002 and 2007 about 78% of annual precipitation contributes to runoff in Hrafndalur. The mean annual discharge-weighted TDS value for the Hrafndalur catchment is 41.3 mg L^{-1} and the mean annual chemical denudation rate (2002–2007) at the outlet of Hrafndalur is 29.3 t km⁻² yr⁻¹ (Table 6) (see above). Surface water chemistry in Hrafndalur is dominated by Cl⁻, Na⁺, Ca²⁺, Si, SO₄²⁻, and Mg²⁺ (Table 8).

The mean annual discharge-weighted suspended sediment concentration at the outlet of the catchment is 13.4 mg L^{-1} and the mean annual yield of suspended sediments is 18.5 t km⁻² yr⁻¹ (Table 6). The mean annual bed-load transport (2002–2007) is

183 t (Table 7). Chemical denudation dominates over the yield of suspended sediments, which makes this rhyolite area different from the extended basalt areas of Austfirðir where yields of suspended sediments are generally higher than chemical denudation rates (Beylich, 1999, 2000). Due to high bed-load transport rates, total mechanical denudation (suspended sediment plus bed-load transport) altogether dominates over chemical denudation in Hrafndalur (Table 9).

Fluvial events in subarctic and arctic landscapes are generated by snowmelt, by rainfall, or by combinations of snowmelt and rainfall. Quantitative studies on the importance of snowmelt- and rainfall-generated peak runoff for annual fluvial sediment transport and fluvial denudation rates in cold environments are still rare (Clark, 1988; Barsch, 1993; Barsch et al., 1994; Gintz and Schmidt, 2000; Beylich, 2000; Beylich and Gintz, 2004; Beylich and Sandberg, 2005). In Hrafndalur runoff peaks occur during snowmelt in spring (normally April-June), during heavy rainfall (especially in fall), and during wintry thaws. A significant increase of suspended sediment concentrations as well as significant bedload transport were measured at days with runoff over 40 mm d^{-1} , with these runoff peaks being caused by heavy rainfall events (Fig. 3). The annual number of days with daily runoff over 40 mm d⁻¹ varied during 2002–2007 between 4 days in 2004 and 2005 and 8 days in 2002 and 2006 (Table 7, Fig. 3). During the six years of investigation, altogether 80% of the annual suspended sediment yield and almost 90% of the annual bed load were transported during these few days with rainfall-generated daily runoff over 40 mm d^{-1} (Table 7, Fig. 3). Significantly increased suspended sediment concentrations as well as high

TABLE 6
Annual suspended sediment yields and annual chemical denudation rates in Hrafndalur (2002-2007).

Year	Precipitation (mm yr ⁻¹)	Runoff (mm yr ⁻¹)	Mean weighted SSC (mg L ⁻¹)	Suspended sediment yield (SSY) (t km ⁻² yr ⁻¹)	Mean weighted TDS (mg L^{-1})	Yield of solutes (t km ⁻² yr ⁻¹)	Chemical denudation (t km ⁻² yr ⁻¹)
2002	2281	1779	17.2	30.6	39.7	70.6	35.9
2003	1666	1283	14.0	18.0	41.0	52.6	26.8
2004	1390	1112	10.2	11.4	42.1	46.8	26.1
2005	1626	1285	10.8	13.8	41.8	53.7	29.3
2006	1869	1458	15.8	23.0	40.9	59.6	31.9
2007	1483	1149	12.2	14.0	42.0	48.3	25.5
Annual mean 2002-2007	1719	1344	13.4	18.5	41.3	55.3	29.3

				2	in a wind normal						
Year	Precipitation (mm yr ⁻¹)	Runoff (mm yr ⁻¹)	Mean weighted SSC $(mg L^{-1})$	Suspended sediment yield (SSY) (t km ⁻² yr ⁻¹)	Number of days with $Q > 40$ (mm d ⁻¹)	SSY during these days $(t \text{ km}^{-2})$	Percent of Total annual SSY (%)	Annual bedload transport (t)	Number of days with $Q > 40$ (mm d ⁻¹)	Bedload transport during these days (t)	Percent of Annual bedload transport (%)
2002	2281	1779	17.2	30.6	∞	23.9	78	321	8	299	93
2003	1666	1283	14.0	18.0	5	14.2	79	150	5	132	88
2004	1390	1112	10.2	11.4	4	9.3	82	129	4	108	84
2005	1626	1285	10.8	13.8	4	11.0	80	120	4	94	78
2006	1869	1458	15.8	23.0	8	17.7	77	237	8	216	91
2007	1483	1149	12.2	14.0	5	11.3	81	138	5	124	90
Annual mean 2002-200	7 1719	1344	13.4	18.5	5.7	14.6	80	183	5.7	162	87

TABLE

bed-load transport rates during these days are connected (i) to debris flows triggered by heavy rainfall and transporting material into main channels and (ii) to slope wash processes. Clearly most sediments transported in the main channels and exported from the Hrafndalur catchment are coming from the slope systems. High mechanical weathering rates in combination with the significant disturbance of the vegetation cover (due to grazing) in larger areas of Hrafndalur are significant factors for the high sediment delivery rates from the slopes during days with heavy rainfall and connected rainfall-generated peak runoff over 40 mm d^{-1} .

Mass Transfers, Sediment Budget, and Trends of **Relief Development**

RELATIVE IMPORTANCE OF THE DIFFERENT DENUDATIVE PROCESSES

On the basis of the process rates calculated for Hrafndalur, the absolute and the relative importance of present-day denudative processes in the entire catchment was estimated by the quantification of the mass transfers caused by the different processes. To allow direct comparison of the different processes, all mass transfers are shown as tonnes \times meters per year (t m yr⁻¹), i.e. as the product of the annually transferred mass and the corresponding transport distance (see Jäckli, 1957; Rapp, 1960; Barsch, 1981; Beylich, 2000, 2008). The mass transfers calculated for Hrafndalur are shown in Table 9. It is stressed that these mass transfers are based on process studies and monitoring carried out over a sixyear period (2002-2007). In computing the mass transfer caused by rock falls and boulder falls, the following assumptions were made: a total debris-supplying rock wall surface of 0.9 km², a mean annual rock wall retreat rate of 3.6 mm yr⁻¹, an average transport distance of 50 m, and an average rock density of 2.2 (Beylich et al., 2007a). The material transported and deposited by avalanches was presumed to have an average density of 2.0. Calculating the mass transfer caused by creep was led by the following assumptions: an affected surface of 1.8 km², an average movement rate of 0.022 m yr^{-1} in a 0.25 m thick layer, and a medium density of 2.0. In contrast to the other denudative processes, chemical denudation affects the total surface of the slope systems. The transport distance of 400 m is about half the medium distance between water divide and main channel.

The transport distance of 1750 m for fluvial solute and suspended sediment transport is about half the distance between the water divide opposite to the catchment outlet and the outlet. The presented estimation of annual mass transfers might include some errors. Anyway, the scale of the calculated mass transfers and the relative importance of the different processes should be correct.

Ranking the different processes according to their annual mass transfers shows that stream work dominates over slope denudation, with fluvial solid transport, including both suspended sediment transport plus bed-load transport, being more important than fluvial solute transport. Rock falls plus boulder falls are clearly the most important slope process, followed by chemical denudation. As a result, according to their relative importance, the different processes can be ranked as follows (see Table 9):

- (1) fluvial suspended sediment plus bed-load transport,
- (2) fluvial solute transport,
- (3) rock falls plus boulder falls,
- (4) chemical slope denudation,
- (5) mechanical fluvial slope denudation (slope wash),
- (6) creep processes,

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 TABLE 8

 Water chemistry in Austfirðir (Hrafndalur and Austdalur).

Samplings 2005 and 2006 $(mg L^{-1})$	Precipitation Hrafndalur (n = 40)	Hrafndalur baseflow $(n = 60)$	Fjardara baseflow $(n = 40)$	Precipitation Austdalur $(n = 40)$	Austdalur baseflow $(n = 60)$
TDS	15.6	54.3	47.7	9.5	34.8
Si	< 0.02	4.92	5.13	< 0.02	3.70
Al	< 0.02	< 0.02	< 0.02	< 0.02	< 0.02
Fe	0.003	0.0052	0.015	0.004	0.063
Mg	0.382	2.31	1.87	0.121	1.41
Ca	0.133	4.99	4.91	0.062	3.09
Na	3.35	8.11	6.90	1.89	5.26
К	<0.5	< 0.5	< 0.5	< 0.5	< 0.5
Mn	< 0.001	0.0012	0.0034	< 0.001	0.0014
Ва	< 0.002	0.0032	< 0.002	< 0.002	< 0.002
Sr	0.002	0.019	0.0107	0.0011	0.0046
F^{-}	< 0.05	0.06	0.07	< 0.05	0.07
Cl ⁻	5.2	9.41	7.45	2.64	7.35
NO ₂ ⁻	< 0.05	< 0.05	< 0.05	< 0.05	< 0.05
NO ₃ ⁻	< 0.05	0.05	0.06	< 0.05	0.07
PO_{4}^{3-}	< 0.2	< 0.2	< 0.2	< 0.2	< 0.2
SO_4^{2-}	0.77	3.64	2.10	0.33	1.39
pH	5.52	7.28	7.34	5.71	7.16

(7) avalanches,

(8) debris flows,

(9) translational slides, and

(10) deflation.

With respect to the temporal variability of process intensities and/or process frequencies, the main snowmelt period (April–June), autumn showing heavy rainfalls (especially September–November) and intensive thaw periods in

TABLE 9

Annual mass transfers by different denudative processes in Hrafndalur (7 km²), as based on investigations carried out from 2001 until 2007 and annual mass transfers by the different denudative processes in Austdalur (23 km²), as based on investigations carried out from 1996 until 2007.

		Volume	Mass	Area	Mass/Area	Average	Mass transfer
	Process	$(m^3 yr^{-1})$	$(t yr^{-1})$	(km ²)	$(t \ km^{-2} \ yr^{-1})$	movement (m)	$(t m yr^{-1})$
		HRAI	FNDALUR (7 km ²))			
Slope denudation	Rockfalls and boulder falls	3195	7029	0.90	7810	50	351,450
	Avalanches	22	44	0.02	2200	234	10,317
	Debris flows	16	31	0.000062	500,000	111	3422
	Translational slides	5	10	0.000043	232,558	26	263
	Creep	450,000	900,000	1.8	500,000	0.022	19,800
	Chemical denudation	92	203	7	29	400	81,200
	Mechanical fluvial denudation	67	133	7	19	400	53,200
	Deflation	0.02	0.04	8	0.005	400	16
Stream work	Solute transport	92	203	7	29	1750	355,250
	Suspended sediment transport	67	133	7	19	1750	232,750
	Bedload transport	83	183	0.18	1017	875	160,125
		AUS	TDALUR (23 km ²)				
Slope denudation	Rockfalls and boulder falls	150	375	1.50	250	100	37,500
	Avalanches	71	142	0.04	3550	290	41,180
	Debris flows	11	22	0.000051	431,373	78	1716
	Translational slides	1.5	3	0.000032	93,750	14	42
	Creep	1,080,000	1,944,000	5.4	360,000	0.01	19,440
	Chemical denudation	74	184	23	8	750	138,000
	Mechanical fluvial denudation	439	966	23	42	750	724,500
	Deflation	0.06	0.12	23	0.005	750	90
Stream work	Solute transport	74	184	23	8	2800	515200
	Suspended sediment	439	966	23	42	2800	2,704,800
	Bedload transport	18	45	0.4	113	500	22,500



FIGURE 4. (a-c) Resistivity tomograms of Wenner surveys along longitudinal profiles at talus cones in Hrafndalur. Dashed lines indicates thickness of talus cones.

winter are periods with comparably high activity of denudative processes.

TRENDS OF RELIEF DEVELOPMENT

The combined analysis of the slope and channel systems, or of slope and stream work respectively, allows statements to be made about trends of relief development in Hrafndalur. Through the retreat of rock faces and the connected formation of talus cones located below these rock faces, slope processes cause a gradually valley widening (Fig. 2). The valley floor is characterized by a fluvial throughput of material delivered from the slope systems. Coupling between slope and fluvial systems exists especially in the upper parts of the catchments where talus cones reach down into the channels. Altogether material transfer from slopes to channels via (i) avalanches, (ii) debris flows, (iii) creep processes, and (iv) fluvial transfers is intensive, with material being fluvially exported from the catchment without longer-term storage within the main channels.

Geoelectrical surveys in the upper parts of Hrafndalur document large volumes of debris with thickness of talus cones

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FIGURE 5. (a-c) Resistivity tomograms of Wenner-Schlumberger surveys along the valley floor in Hrafndalur. (a) Profile i in Figure 1. (b) Profile j in Figure 1. (c) Profile c in Figure 1.

of about 15-20 m (Fig. 4; see above) and permit inferring the Holocene valley filling and regolith thickness in the lower parts of the catchment using geoelectrical surveys (Fig. 5). The generally high conductivity within the subsurface can be related to the high porosity and water storage capacity of the weathered rhyolite. Due to indistinct resistivity contrasts, the differentiation between regolith and bedrock is made difficult (Fig. 5). It seems that the boundary between bedrock and overlying regolith/valley fill was not reached with the applied survey length, which was limited to 175 m due to the acquisition configuration. This indicates that the depth of the boundary between bedrock and regolith/valley fill is greater than 25 m. The high porosity of the rhyolite and the large regolith thicknesses found in lower parts of Hrafndalur explain the comparably high rates of chemical slope denudation, with the values in this rhyolite area clearly exceeding the rates of chemical slope denudation in basalt areas of Austfirðir (Table 9) (Beylich, 2000, 2003, 2007). The large volumes of postglacial valley fillings in the lower part of Hrafndalur indicate high rates of fluvial particulate transport.

Due to the comparably high mechanical weathering and sedimentary transfer rates, postglacial modification of the Pleistocene glacial landscape is clearly more advanced than in the extended basalt areas of Austfirðir. Anyway, due to the short time since the deglaciation (about 8000 yr), there has been no adjustment of the glacially formed landforms to the geomorphic processes, which have been operating under Holocene morphoclimates.

Comparison of the results from Hrafndalur with data collected in the Austdalur catchment, located in the basalt area to the south (Fig. 1, Table 9) (see also Beylich, 1999, 2000, 2003, 2007), reveals significant differences between the two catchments. Both areas are similar with respect to morphoclimate, vegetation, human impact, and topography, but significantly different with respect to lithology. The rhyolites in Hrafndalur are clearly less resistant than the basalts found in Austdalur (Beylich et al., 2007a), and postglacial modification of the Pleistocene glacial landforms is limited in Austdalur, but significant in Hrafndalur (Beylich, 2000, 2007, accepted). The high level of coupling between slope and channel systems found in Hrafndalur does not exist to the same extent in Austdalur, and results in a lower contribution to bed-load transport in Austdalur. The intensive coupling between slope and fluvial systems in Hrafndalur is strongly connected to the Holocene development of extended talus cones, which partly reach and directly feed into the main channels. By comparison, the Austdalur slope systems are only connected to the main channels via small creeks that drain slopes (Beylich, 1999, 2000). Hence, slope and channel systems are largely decoupled in this catchment.

Suspended sediment yields in Hrafndalur are clearly lower than in Austdalur and chemical denudation dominates over suspended sediment yields in the Hrafndalur catchment, while the reverse is the case in Austdalur (Table 9). These differences between the two areas can be explained by the significantly higher porosity and connected higher infiltration and water storage capacity of the rhyolites in Hrafndalur as compared to the basalts in Austdalur (see above, Beylich et al., 2007a).

Conclusions

Combined quantitative measurements of the relevant denudative slope processes and stream fluxes in the 7 km^2 catchment Hrafndalur situated in a rhyolite area in the northern part of the Icelandic Eastern Fjord region (Austfirðir) provide information on the absolute and relative importance of the different denudative processes and Holocene storage element volumes.

The selected catchment-based approach was effective in analyzing the sediment budget and trends of postglacial relief development in a selected cold environment with given environmental setting. Additional studies carried out with unified geomorphic field methods (Beylich and Warburton, 2007; Beylich et al., 2007b, 2008; Lamoureux et al., 2007) in cold environments having different morphoclimatic, vegetational, topographic, lithological/geological, and tectonic features will help to gain better understanding of the internal differentiation of high-latitude and high-altitude cold environments (Barsch, 1984, 1986). Furthermore, additional information on the control mechanisms of processes, the role of extreme geomorphic events for longer-term mass transfers and sediment budgets, the intensity of geomorphic processes in cold environments, and the relative importance of different processes for slope and valley formation and relief development under different cold environment conditions can be assessed.

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